

Continuum mechanics & Fundamentals

1. Tensors & index notation

vectors $\underline{a} \cdot \underline{b} = a_i b_i$ rule = repeated indices sum
 $= a_1 b_1 + a_2 b_2 + a_3 b_3 \quad i \in \{1, 2, 3\}$

tensor $\underline{A} : \underline{B} = A_{ij} B_{ij}$

tensor $(\underline{bn})_i = \underline{b}_{ij} n_j$
vector free index

$\underline{ijk}$ jki kij

Kronecker delta $\delta_{ij} = \begin{cases} 1 & i=j \\ 0 & i \neq j \end{cases}$

$\epsilon_{ijk} = \begin{cases} 1 & \text{Even perm.} \\ 0 & \text{repeated} \\ -1 & \text{odd perm} \end{cases}$

$\delta_{ij} a_j = a_i$

$\epsilon_{112} = 0$

Cross product $(\underline{a} \times \underline{b})_i = \epsilon_{ijk} a_j b_k$
free

$$(\nabla u)_{ij} = \frac{\partial u_i}{\partial x_j} = u_{i,j}$$

$$\nabla \cdot u = \frac{\partial u_i}{\partial x_i} = u_{i,i}$$

$$(\nabla \cdot b)_i = b_{ij,j}$$

2nd tensor
order

$$A = A^T \quad \text{symmetric} \quad A_{ij} = A_{ji}$$

$$-A = A^T \quad \text{antisymmetric} \quad A_{ij} = -A_{ji}$$

$$A = \underbrace{\frac{A + A^T}{2}}_{A_s} + \underbrace{\frac{A - A^T}{2}}_{A_a}$$

Proposition : $S = S^T, A = -A^T \Rightarrow A:S = 0$

Pf.

$$A:S = \underbrace{A_{ij} S_{ij}}_{=} = \underbrace{A_{ji} S_{ji}}_{=} = - \underbrace{A_{ij} S_{ij}}_{=}$$

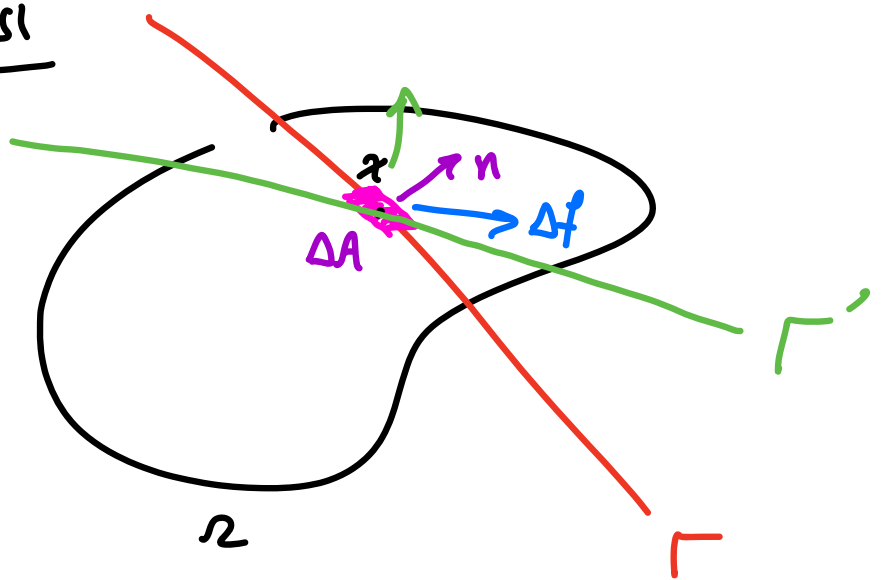
$$A_{ij} S_{ij} = 0$$

□

Corollary : $S = S^T$, A any 2nd-order tensor

$$\Rightarrow S:A = S:A_s + \underbrace{S:A_a}_{=0}$$

2. Traction & Stress



$$t(x, n) = \lim_{\Delta A \rightarrow 0} \frac{\Delta f}{\Delta A}$$

Thm. Cauchy Stress

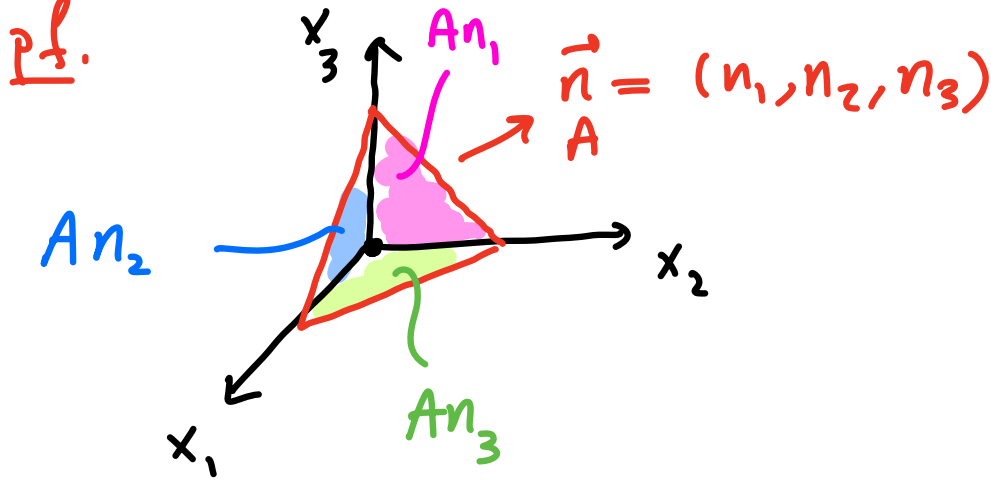
Traction depends linearly on $\vec{n}$:

$$t(n) = \underline{\sigma} n$$



exists (Cauchy Stress)

$$t_i = \underline{\sigma}_{ij} \underline{n}_j$$



$$-t(e_1) \cancel{An_1} - t(e_2) \cancel{An_2} - t(e_3) \cancel{An_3} \quad o(h^2)$$

$$+ t(n) \cancel{A} + \underbrace{\vec{b} \cdot \vec{v}}_{o(h^3)} = \cancel{mv} \underbrace{a}_{\text{acceleration}} \quad o(h^3)$$

$$\left. \begin{array}{l} A \propto h^2 \\ v \propto h^3 \end{array} \right\}$$

so, $h \rightarrow 0$

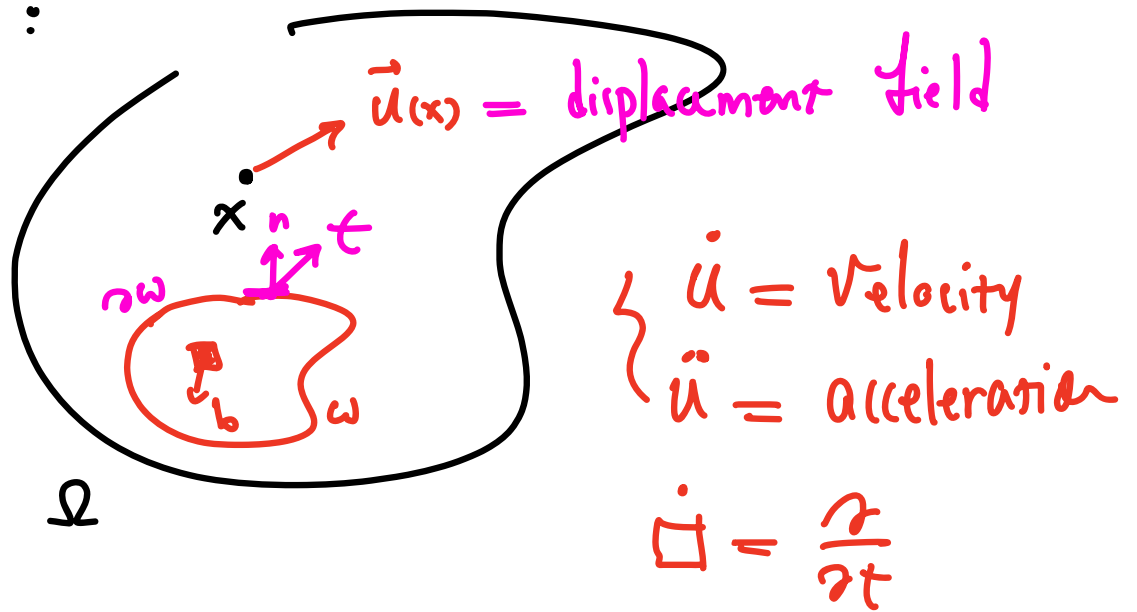
$$t(n) = t(e_i) \underbrace{n_i}_{\vec{b}}$$

□

3. Balance laws

Thm. Balance of linear momentum $\Leftrightarrow \dot{v} \cdot b + b = \dot{J} \ddot{u}$

side note :



pt. Pick subbody $\omega \subseteq \Omega$, then:

$$\cancel{\frac{d}{dt}} \int_{\omega} \rho \ddot{u} dV = \int_{\partial\omega} \underbrace{t}_{bn} dA + \int_{\omega} b dV \quad \leftarrow$$

$$\rho dV = \text{const.} \quad (\text{due to Cauchy Thm})$$

$$\int_{\omega} \rho \ddot{u} dV = \underbrace{\int_{\partial\omega} bn dA}_{\parallel} + \int_{\omega} b dV$$

$$\int_{\omega} \nabla \cdot b dV$$

$$\rightarrow \int_{\omega} (\underbrace{f\ddot{u} - \nabla \cdot b - b}_0) dV = 0 \quad \forall \omega \subseteq \Omega$$

Thm. Balance of angular momentum
 $\Leftrightarrow \boxed{b = b^T}$

pf. Pick $\omega \subseteq \Omega$

$$\cancel{\frac{d}{dt}} \int_{\omega} x \times \underbrace{f\ddot{u}}_{\checkmark} dV = \overbrace{\int_{\partial\omega} x \times \underbrace{t}_{\checkmark} dA}^{\star} + \int_{\omega} x \times \underbrace{b}_{\checkmark} dV$$

$$\frac{d}{dt} (x \times \dot{u}) = \underbrace{\dot{u} \times \dot{u}}_0 + \underbrace{x \times \ddot{u}} \quad \dot{x} = \dot{u}$$

$$[x \times (bn)]_i = \epsilon_{ijk} x_j \underbrace{(bn)_k}_{\checkmark} = \dots$$

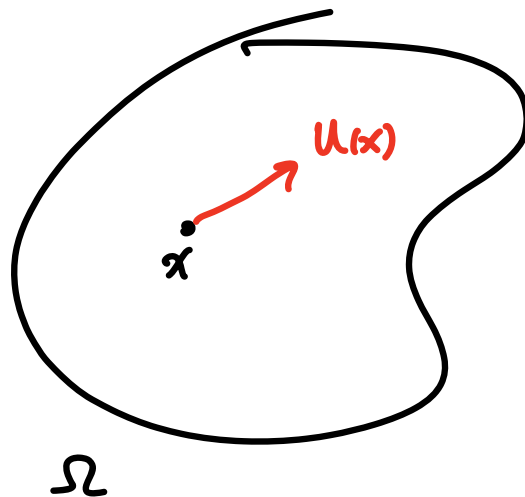
$$\star = \int_{\omega} x \times \nabla \cdot b + \underbrace{\int_{\omega} \epsilon_{ijk} \delta_{kj}}_{\checkmark} \quad \forall \omega$$

Corollary: Because $\delta = \delta^T \Rightarrow$

$$\delta = \sum_{i=1}^3 \delta_i \cdot \underset{\substack{\uparrow \\ \text{Spectral decomp.}}}{n_i} \otimes n_i$$

$$A = A^T \rightarrow A = \underset{\substack{\text{"} \\ \begin{pmatrix} | & | & | \\ u_1 & u_2 & u_3 \\ | & | & | \end{pmatrix}}}{P} \Lambda \underset{\substack{\text{"} \\ \begin{pmatrix} \lambda_1 & 0 & 0 \\ 0 & \lambda_2 & 0 \\ 0 & 0 & \lambda_3 \end{pmatrix}}}{P^T} \rightarrow A = \sum_{i=1}^3 \lambda_i \underbrace{u_i^T u_i}_{\text{---}}$$

4. Strain



Definition:

$$\epsilon = \nabla_s u = \frac{\nabla u + \nabla u^T}{2} = \text{sym } \nabla u$$

$$\epsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i}) \quad \epsilon_{11} = \frac{\partial u_1}{\partial x_1}$$

$$\nabla u = \underline{\epsilon} + \omega$$

Strain " "

$$\epsilon_{12} = \frac{1}{2} \left(\frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right)$$

$$\frac{\nabla u - \nabla u^T}{2}$$

responsible for
solid-body rotations
≠ deformation

Definition: Volumetric-Deviatoric split

$$\left\{ \begin{array}{l} \sigma = \sigma_m + \mathcal{S} \\ \text{"volumetric"} \quad \text{"deviatoric"} \end{array} \right.$$

Stress

$$\left\{ \begin{array}{l} \epsilon = \epsilon_v + e \\ \text{"volumetric"} \quad \text{"deviatoric"} \end{array} \right.$$

Strain

$$\sigma_m \equiv \frac{\text{tr}(\sigma)}{3} \mathbf{1}$$

"pressure"

$$\sigma = \begin{pmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{21} & \sigma_{22} \end{pmatrix} \quad \underline{\underline{2D}}$$

$$\text{tr}(\sigma) = \sigma_{11} + \sigma_{22}$$

$$\mathcal{S} \equiv \sigma - \sigma_m \mathbf{1}$$

$$\mathbf{1} = \begin{pmatrix} 1 & 0 \\ 0 & 1 \end{pmatrix}$$

$$\epsilon_v \equiv \text{tr}(\epsilon) \mathbf{1}$$

$$\nabla \cdot u = \begin{cases} + \text{expansion} \\ - \text{contraction} \end{cases} \quad e = \begin{pmatrix} \epsilon_{11} + \epsilon_{22} \end{pmatrix}$$

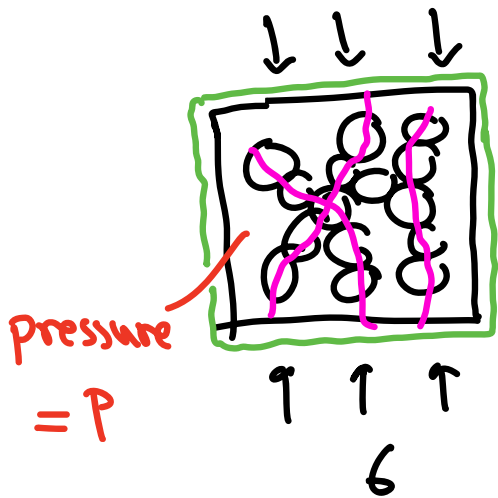
$$e \equiv \epsilon - \epsilon_v$$

$$\begin{cases} \epsilon_{11} = \frac{\partial u_1}{\partial x_1} \\ \epsilon_{22} = \frac{\partial u_2}{\partial x_2} \end{cases}$$

Remarks :

① Sign Convention

$$\sigma_m = \begin{cases} + \text{tension} \\ - \text{compression} \end{cases}$$



$$\sigma = \sigma' - b P$$

total stress effective stress (solid) pore pressure

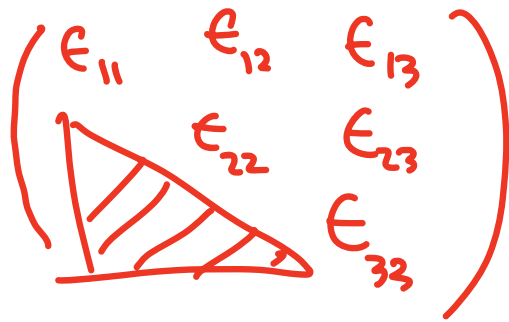
b/c $P > 0$

Biot Coefficient
(=1 soils
Terzaghi)

Ex: $\sigma = -100 \text{ Pa}$
 $\sigma' = -60 \text{ Pa}$
 $\rightarrow P = 40 \text{ Pa}$

② Specifying $u(x) \rightarrow \epsilon(x) = \nabla_s u$ is well-defined

Specifying $\epsilon(x) \longrightarrow u(x) \stackrel{!}{=} \int \epsilon(x)$ not well-defined



6 components)

overdetermined
 $\Rightarrow$ need constraints

Saint-Venant Constraints

$$\nabla_x (\nabla_x \epsilon) = 0$$

5. Linear Elasticity

Define. A material is hyperelastic if:

$$\sigma = \frac{\partial \Psi}{\partial \epsilon}$$

Ψ = elastic energy
density / volume.

Ex: linear elasticity

$$\Psi = \frac{1}{2} \epsilon : \mathbb{C} : \epsilon$$

stiffness tensor (4th order)

$$\Psi = \frac{1}{2} \epsilon_{ij} \mathbb{C}_{ijkl} \epsilon_{kl} \rightarrow \frac{\partial \Psi}{\partial \epsilon} = \mathbb{C} : \epsilon = \underline{\underline{\sigma}}$$

$$\sigma_{ij} = \frac{\partial \Psi}{\partial \epsilon_{ij}}$$

$$\frac{\partial \Psi}{\partial \epsilon_{ij}} = \dots \frac{\partial \epsilon_{kl}}{\partial \epsilon_{ij}} = \delta_{ki} \delta_{lj}$$

Proposition . Minor Symmetry

$$\mathbb{C}_{ijkl} = \mathbb{C}_{jikl} = \mathbb{C}_{ijlk}$$

pf. $\sigma = \sigma^T, \epsilon = \epsilon^T$



$$\sigma_{ij} = \sigma_{ji} \rightarrow \mathbb{C}_{ijkl} \epsilon_{kl} = \mathbb{C}_{jikl} \epsilon_{kl} \quad \forall \epsilon_{kl}$$

Proposition . Major Symmetry

$$\mathbb{C}_{ijkl} = \mathbb{C}_{klij} \iff \text{linear (hyper)elastic}$$

$$\sigma = \frac{\partial \Psi}{\partial \epsilon} = \underline{\underline{\mathbb{C} : \epsilon}}$$

pt. ($\Leftarrow$) ~~$\psi = \frac{1}{2} \epsilon : \mathbb{C} : \epsilon$~~ $\rightarrow \boxed{\psi = \mathbb{C} : \epsilon} \quad \checkmark$
 $= \frac{\partial \psi}{\partial \epsilon}$

$$\rightarrow \mathbb{C}_{ijkl} = \frac{\partial^2 \psi}{\partial \epsilon_{ij} \partial \epsilon_{kl}} = \mathbb{C}_{klij}$$

$$(\Rightarrow) \psi = \frac{1}{2} \epsilon : \psi : \epsilon$$

$$\frac{\partial \psi}{\partial \epsilon_{ij}} = \frac{1}{2} \left(\mathbb{C}_{ijkl} + \mathbb{C}_{klij} \right) \epsilon_{kl} \Rightarrow \square$$

Proposition. Parameter Count of $\mathbb{C}$

$\mathbb{C}_{ijkl}$ has at most 21 constants.
 $3 \times 3 \times 3 \times 3 = 3^4 = 81$

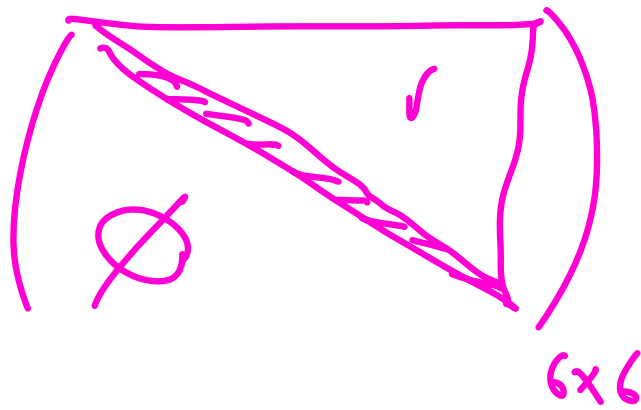
pt.

$$\mathbb{C}_{ij \quad kl}$$

$$ij = \begin{pmatrix} 1,1 & 1,2 & 1,3 \\ \hline 2,2 & 2,3 & \\ 3,3 & & \end{pmatrix}$$

pairs

$$(ij)(kl) =$$



$$\frac{6^2 - 6}{2} + 6 = 21$$

linear isotropic stiffness :

$$C_{ijkl} = \underbrace{1}_{\text{Bulk?}} \delta_{ij} \delta_{kl} + \underbrace{\mu}_{\text{shear}} (\delta_{ik} \delta_{jl} + \delta_{il} \delta_{jk})$$

Lame' Constants

END of Slot 1

Variational Calculus

1. Functionals & 1st variations

$$\text{function } x \mapsto f(x)$$

$$\mathbb{R} \rightarrow \mathbb{R}$$

$$\mathbb{C} \rightarrow \mathbb{C}$$

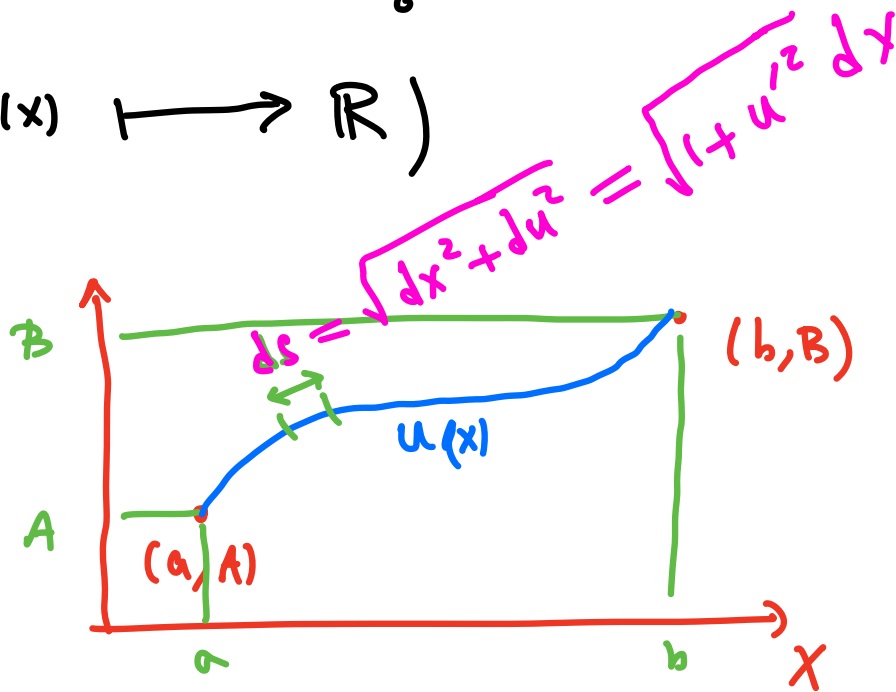
Functional $J(x) \mapsto \mathbb{R}$ or $\mathbb{C}$

Define.

$$J[u] = \int_0^L F(x, u, u') dx$$

$$(u(x) \mapsto \mathbb{R})$$

Ex:

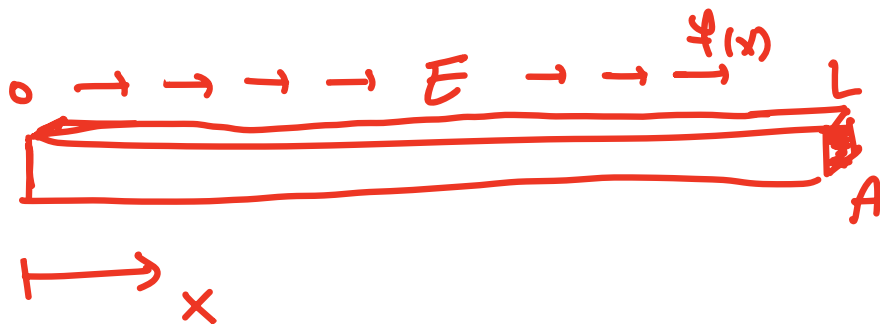


$$L[u] = \int_0^L ds = \int_0^L \sqrt{1 + u'^2} dx$$

$$\cancel{F(x, u, u')} =$$

Ex: $\Pi[u] = \int_0^L (\text{Elastic potential}) - (\text{work done})$ virtual

$$\frac{1}{2} EA u'^2 - \gamma u$$



$$\epsilon = \frac{du}{dx} = u' \quad \delta = E\epsilon = Eu'$$

$$\Psi = \frac{1}{2} \epsilon : \mathbb{C} : \epsilon = \frac{1}{2} E u'^2$$

$$\Pi[u] = \int_0^L (\Psi A - f u) dx$$

Def. First Variation

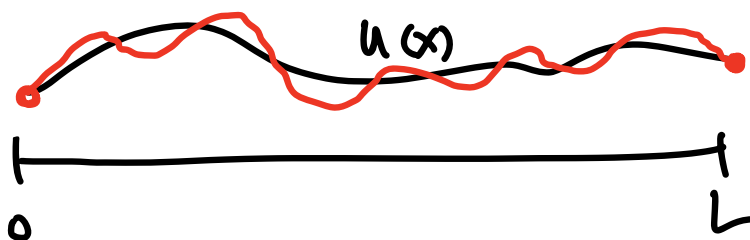
$$\delta J \equiv \frac{d}{d\epsilon} J[u + \underbrace{\epsilon \eta}_{\text{perturbation}}] \Big|_{\epsilon=0}$$

perturbation

$\eta(x)$ function (Pick/Smooth)

ϵ small numbers

Note: The picked $\eta(x)$ must be admissible



$$\text{BCs} \begin{cases} u(0) = u_0 \\ u(L) = u_L \end{cases} \longrightarrow \tilde{u} = u + \underline{\epsilon \eta}$$

$$\eta(0) = \eta(L) = 0$$

Thm. The variation of J is :

$$\delta J = \int_0^L \left(F_u - \frac{d}{dx} F_{u'} \right) \eta + F_{u'} \eta \Big|_0^L$$

$$\begin{cases} F_u = \frac{\partial F}{\partial u} \\ F_{u'} = \frac{\partial F}{\partial u'} \end{cases}$$

pf.

$$\delta J = \frac{d}{d\epsilon} J[u + \epsilon \eta] \Big|_{\epsilon=0}$$

$$= \frac{d}{d\epsilon} \int_0^L F(x, u + \underline{\epsilon \eta}, u' + \underline{\epsilon \eta'}) dx \Big|_{\epsilon=0}$$

$$\begin{aligned}
&= \underbrace{\int_0^L F_u \eta}_{\text{I}} + \underbrace{\int_0^L F_{u'} \eta'}_{\text{II}} \\
&\quad \underbrace{F_{u'} \eta}_{\text{I}} - \underbrace{\int_0^L \frac{d}{dx} F_{u'} \eta}_{\text{II}} \quad \square
\end{aligned}$$

2. Euler-Lagrange Equations

Lemma. Fundamental lemma

if $g(x)$ is continuous and
(η sufficiently smooth)

$$\eta(0) = \eta(L) = 0$$

$$\int_0^L g \eta = 0 \quad \forall \eta$$

$$\Rightarrow g = 0 \quad \text{in } x \in [0, L]$$

Thm. Euler-Lagrange Equations

$$\delta J = 0 \quad \forall \eta$$

$$\eta(0) = \eta(L) = 0$$

$$\Rightarrow$$

$$\boxed{F_u - \frac{d}{dx} F_{u'} = 0}$$

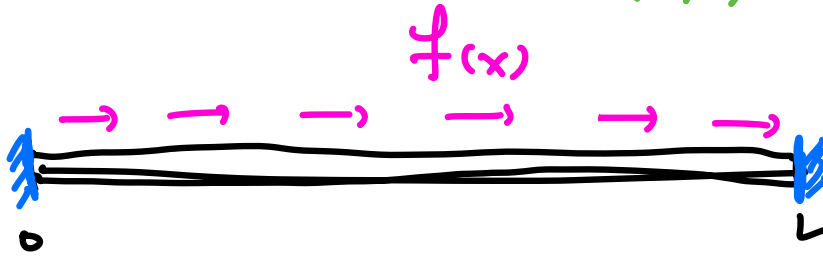
Ex. Axially loaded bar

$$\Pi[u] = \int_0^L \left(\underbrace{\frac{1}{2} EA u'^2 - f u}_{F(x, u, u')} dx$$

$$u(0) = u(L) = 0$$

BCs

$$\eta(0) = \eta(L) = 0$$



$$\delta \Pi = \underbrace{F_u}_{-f} - \frac{d}{dx} \underbrace{F_{u'}}_{EA u'} = - (EA u')' - f$$

$$= \boxed{- EA u'' - f = 0}$$

if E, A Constant

$$\downarrow \quad \downarrow$$

$$v.b + f = 0$$

Question : Does a BVP have a functional "parent" ? Not always, must be compatible with BCs

δ -rule :

- δ commutes w/ $\vec{\nabla}$ and $\frac{\partial}{\partial x}$ ($\delta \epsilon = \vec{\nabla}_s \delta u$)
- Chain rule applies : $\delta F(u, u') = F_u \delta u + F_{u'} \delta u'$

NB. $\delta u = \cancel{\epsilon} \eta$

- $\delta(\text{fixed function}) = 0$ (Ex: $\delta b = 0$)

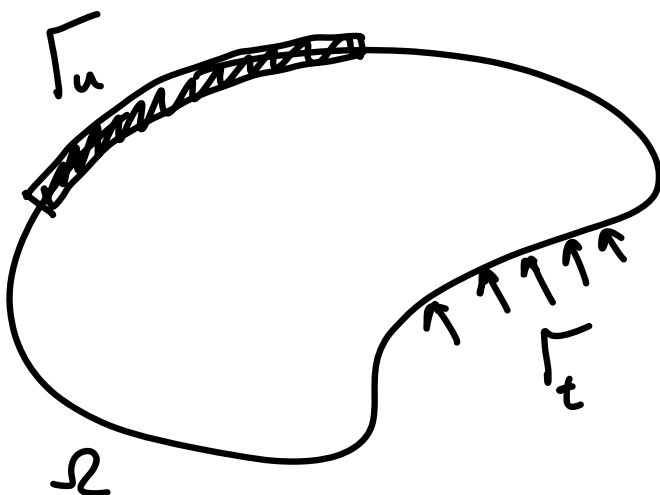
$$\delta \delta u = 0$$

$$\eta'' = \text{fixed}$$

3. Essential and natural BCs

- Essential $\equiv$ Dirichlet BCs

$\Rightarrow$ imposed on the admissible set of variations



$$\partial\Omega = \Gamma_u \cup \Gamma_t$$

Admissible $\eta(x)$

$$\eta(x) = 0 \quad \forall x \in \Gamma_u$$

- Natural BCs $\Rightarrow$ show up in the functional itself

Proposition :

$$J[u] = \int_0^L F(x, u, u') dx$$



$$u(x=0) = \bar{u} \longrightarrow \eta(0) = 0$$

at $x=L$ free $\longrightarrow$ BC imposed here?

$$\Rightarrow \delta J = \int_0^L \left(F_u - \frac{d}{dx} F_{u'} \right) \eta + F_{u'} \eta \Big|_{x=L}$$

$$\text{if } \delta J = 0 \Rightarrow \left\{ F_u - \frac{d}{dx} F_{u'} = 0 \right.$$

$$\left. F_{u'} \Big|_{x=L} = 0 \right\}$$

Boundary Cond.
at $x=L$!!

must be compatible

① Pf. First pick η st. $\eta(0) = \eta(L) = 0$

$\Rightarrow$ boundary term $= 0$

$$\boxed{F_u - \frac{d}{dx} F_{u'} = 0} \quad \star$$

② Having shown $\star$, Pick now η that is free at $x=L$

$$\text{from } \star \rightarrow \delta J = F_{u'} \eta \Big|_{x=L} \rightarrow \boxed{F_{u'} \Big|_{x=L} = 0}$$

arbitrary specify force

Ex: Consider the BC: $\boxed{F_{u'}(x=L) = P \neq 0}$

WANT Constant force

$$F_{u'} = EA u' \text{ force}$$

$\rightarrow EA u' = P \rightarrow u' = \#$ Neumann
/// natural

This BC clashes/incompatible with

$$\delta J = 0 \quad \left(J = \int_0^L F dx \right)$$

Q: BVP $\left\{ \begin{array}{l} EAu'' + f = 0 \quad \checkmark \\ u(x=0) = \bar{u} \\ F_u(x=L) = P \neq 0 \quad \checkmark \end{array} \right. \rightarrow \text{functional "parent"}$

if $\hat{J}[u] = \int_0^L F dx - P u_L \Rightarrow \delta \hat{J} = 0$

"parent"

Compatible

1st.

$$\delta \hat{J} = \int_0^L \delta F dx - P \delta u_L$$

$$F_u \delta u + F_{u'} \delta u'$$

$$0 = \delta \hat{J} = \int_0^L \underbrace{\left(F_u - \frac{d}{dx} F_{u'} \right)}_{=0} \delta u + \underbrace{(F_{u'} - P)}_{=0} \delta u_L$$

Thm. Non-Dirichlet BCs

$$J[u] = \int_0^L F dx + B(u_L) \quad u(0) = \bar{u}$$

Then

$$\delta J = 0 \Rightarrow \left. \begin{array}{l} \text{E-L eq.} \\ \end{array} \right\}$$

$$\left. \begin{array}{l} F_u \Big|_{x=L} + \underbrace{B'(u_L)}_{-P} = 0 \end{array} \right\}$$

Ex.

$$B(u_L) = -P u_L$$

4. Functional w/ higher derivative

$$J[\omega] = \int_0^L \underline{F(x, \omega, \omega', \omega'')} dx \quad x \in [0, L]$$

$$\underline{\delta J = 0} \Rightarrow \left[F_\omega \ominus \underline{\frac{d}{dx}} F_{\omega'} \oplus \underline{\frac{d^2}{dx^2}} F_{\omega''} = 0 \right]$$

$$\left. \begin{array}{l} \textcircled{1} \quad \omega = \bar{\omega} \quad \text{or} \quad F_{\omega'} - (F_{\omega''})' = 0 \\ \textcircled{2} \quad \omega' = \bar{\omega}' \quad \text{or} \quad F_{\omega''} = 0 \end{array} \right\}$$

Pf.

$$\delta J = \int_0^L \delta F = \int_0^L \left(F_\omega \delta \omega \cancel{-} F_{\omega'} \delta \omega' \oplus F_{\omega''} \delta \omega'' \right) dx$$

$\xleftarrow{x1} \textcircled{1}$ $\xleftarrow{x2} \textcircled{2}$
